# TRAFFIC SIGNAL ANALYSIS ON WASHINGTON BOULEVARD BETWEEN FIDELIA AVENUE AND I-5 FREEWAY

# Prepared for:

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### **EXECUTIVE SUMMARY**

This Traffic Signal Analysis is prepared in response to request from Staff to evaluate the potential impacts of a new traffic signal on Washington Boulevard between Fidelia Avenue and the I-5 (Santa Ana Freeway) Southbound On/Off-Ramps in the City of Commerce. This analysis will include discussions on the traffic signal warrants, intersection delay, queue lengths, and additional improvements at the intersection. The results of this analysis will demonstrate that the new traffic signal will not significantly impact the traffic operation and vehicular flow on Washington Boulevard during the AM and PM peak hours.

### INTRODUCTION

The new traffic signal is proposed to be located on Washington Boulevard approximately 690 feet east of Fidelia Avenue and approximately 515 feet west of the I-5 (Santa Ana Freeway) Southbound On/Off-Ramps. It will provide signal control and full access for Commerce Square and the Celluphone building (6119 Washington Boulevard) to the north and Arco gas station to the south (see Attachment 1).

### **EXISTING CONDITIONS**

The westerly driveway for Commerce Square (Driveway 1) is a full access driveway, the shared driveway for Commerce Square and the Celluphone building (Driveway 2) is a full access driveway, the Arco gas station driveway (Driveway 3) is a full access driveway, and the driveway for the Celluphone building (Driveway 4) is a right-turn in/out and left-turn in restricted access driveway. Washington Boulevard is currently under construction from Indiana Street to the I-5 Santa Ana Freeway to provide three through lanes for the eastbound and westbound directions. Construction started in January 2015 and it is anticipated to be complete in early 2017.

### PROJECT IMPACT EVALUATION

As part of the Washington Boulevard improvements, Driveway 3 will be relocated to the west to align with Driveway 2. The project will also construct a raised center median with Driveway 1 becoming a right-turn in/out restricted driveway, Driveway 2 becoming a right-turn in/out and left-turn in restricted driveway, and Driveway 4 becoming a right-turn in/out restricted driveway. The new traffic signal is proposed to be implemented after the completion of the street improvements and will provide full access to Driveway 2/Driveway 3. Without the new traffic signal, all outbound left-turn traffic will have to instead turn right onto Washington Boulevard then make an U-turn at the Fidelia signal. The additional U-turn traffic will negatively impact the operation of the Fidelia signal because U-turn traffic require additional time to complete the movement compared to left-turn traffic. The new traffic signal will also provide control and improve traffic safety at Driveway 2/Driveway 3 for the uncontrolled left-turn movements crossing 3 lanes of traffic. The left-turn movements will be served by a 90-foot eastbound lane with a 90-foot transition and a 55-foot westbound lane with a 90-foot two-way left-turn lane.

### A. Traffic Counts

In order to determine the traffic volumes on Washington Boulevard between Fidelia Avenue and the I-5 Freeway, vehicle turning movement counts at the driveways and arterial machine counts were conducted on Tuesday, October 14, 2014 and on Wednesday, October 15, 2014 for 48 hours. Additional vehicle turning movement counts were conducted for the Fidelia Avenue and Washington Boulevard intersection on Tuesday, December 23, 2014 from 7 AM to 9 AM and from 3PM to 6 PM, including a westbound left-turn lane queuing study. A copy of the counts data is included in Appendix A. The arterial machine counts show that the AM peak hour is at 7 AM and the PM peak hour is at 4 PM. It should be noted that these counts from 2014 are still valid as the area has been under construction and the ambient growth is expected to be low.

### B. Traffic Signal Warrants

Using the traffic counts for Washington Boulevard, traffic signal warrants have been prepared for the Driveway 2/Driveway 3 and Washington Boulevard intersection to determine if a traffic signal is warranted. The warrants are based on worksheets contained in the California Manual on Uniform Traffic Control Devices 2014 Edition Revision 1 and a copy is included in Attachment 2. As discussed previously, Driveway 1 and Driveway 4 will become right-turn in/out restricted access drives after the construction on Washington Boulevard is complete. Therefore, the left-turn inbound and outbound volumes at these two driveways are added to the corresponding left-turn movements at Driveway 2. The worksheets show that the traffic volumes at this intersection satisfies Warrant 1, Warrant 2, and Warrant 3. Therefore, the new traffic signal is justified.

# C. Intersection Delay and Queue

To quantify the delay that motorists will experience at this new traffic signal, a model of this intersection has been prepared using Synchro 8 for the AM peak hour and the PM peak hour. Consistent with what was done for the traffic signal warrants, the left-turn inbound and outbound volumes at Driveway 1 and Driveway 2 are added to the corresponding left-turn movements at Driveway 2. A copy of the reports are included in Attachment 3. As shown in the reports, during the AM peak hour the northbound direction experiences 10 seconds of delay, the eastbound direction experiences 7 seconds of delay, the southbound direction experiences 0.2 seconds of delay, and the westbound direction experiences 6 seconds of delay. Overall this intersection experiences 6.5 seconds of delay, which equates to Level of Service A. During the PM peak hour, the northbound direction experience 10.5 seconds of delay, the eastbound direction experiences 5.9 seconds of delay, the southbound direction experiences 6.3 seconds of delay, and the westbound direction experiences 4 seconds of delay. Overall this intersection experiences 5.3 seconds of delay, which equates to Level of Service A. With the implementation of the proposed signal, the delay experienced by the eastbound and westbound movements on Washington Boulevard will increase. However, as discussed above and shown in Attachment 3, even with the additional delay the proposed signal is still projected to operate at Level of Service A during the AM and PM peak hours. In addition by providing a left-turn out access, the number of vehicles making an U-turn at the intersection of Fidelia Avenue and Washington Boulevard will decrease.

Also shown in the reports, during the AM peak hour the 95<sup>th</sup> percentile queue for the eastbound through direction is 80 feet with 32 feet for the eastbound left-turn direction and the westbound through direction is 127 feet with 79 feet for the westbound left-turn direction. The 90-foot eastbound left-turn lane with a 90-foot transition and the 55-foot westbound left-turn lane with a 90-foot two-way left-turn lane can accommodate the projected queue lengths. During the PM peak hour the 95<sup>th</sup> percentile queue for the eastbound through direction is 170 feet with 25 feet for the eastbound left-turn direction and the westbound through direction is 85 feet with 37 feet for the westbound left-turn direction. Again, the eastbound and westbound left-turn lanes can accommodate the projected queue lengths. The distance from the eastbound limit line of the new traffic signal to the crosswalk of the Fidelia signal is approximately 640 feet. The 170 feet of eastbound through movement queue during the PM peak hour will not impact the operation of the Fidelia signal. The distance from the westbound limit line of the new traffic signal to the crosswalk of the signal at the I-5 SB On/Off-Ramps is approximately 460 feet. The 127 feet of westbound through movement queue during the AM peak hour will not impact the operation of the ramp signal.

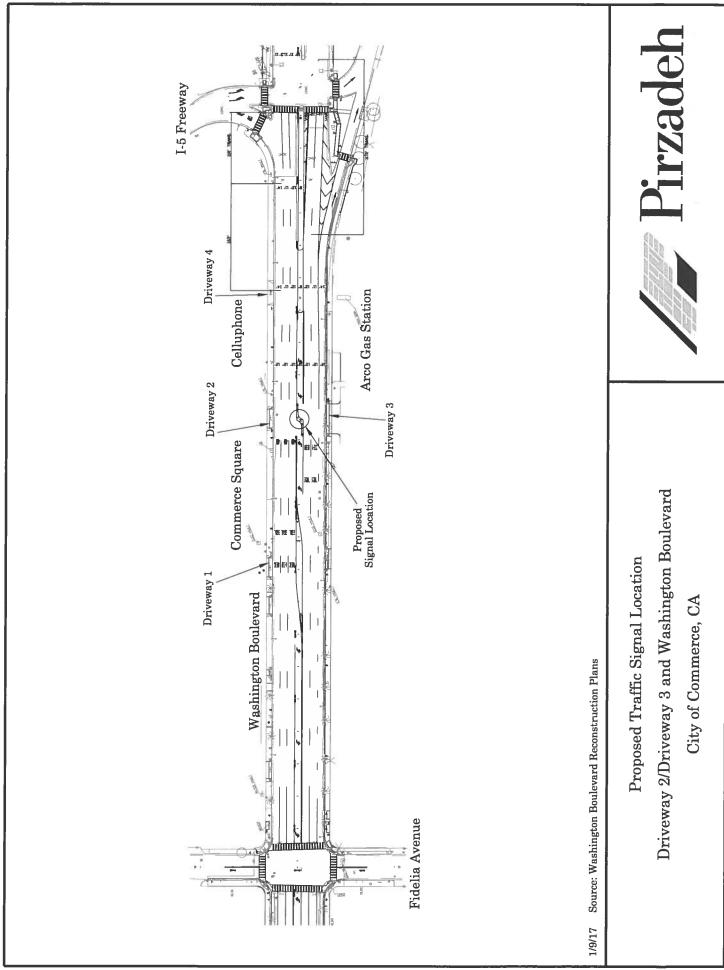
### ADDITIONAL IMPROVEMENTS

The other goal of the Washington Boulevard improvements is to enhance the beautification of this arterial through the implementation of landscaping along the curb and in the raised center medians. The raised center median between the westbound left-turn lane at Fidelia and the eastbound left-turn lane at Driveway 2 will be landscaped, the median between the eastbound left-turn lane and the westbound left-turn lane at Driveway 2 will not be landscaped, and the median between the westbound left-turn lane at Driveway 2 and Caltrans Right-of-Way will be landscaped. The new traffic signal at Driveway 2/Driveway 3 and Washington Boulevard will remove the raised center median but this median will not be landscaped. In addition, with the implementation of the signal, the westbound left-turn lane can be modified to a 100-foot lane with a 45-foot transition. The 45 feet of transition will increase the raised center median area on Washington Boulevard and allow the installation of additional groundcover and up to 3 palm trees. Therefore, the new traffic signal will have the added benefit of increasing the raised center median landscaping which is consistent with the street beautification goal.

### CONCLUSIONS

As documented above, multiple traffic counts have been conducted at the driveways and the Washington Boulevard arterial. Using the data from these counts, traffic signal warrants were conducted based on California Manual on Uniform Traffic Control Devices 2014 Edition Revision 1 requirements. The warrants show that the Driveway 2/Driveway 3 and Washington Boulevard intersection satisfies Warrant 1, Warrant 2, and Warrant 3. Therefore, the installation of a new traffic signal at this intersection is warranted. Also using these counts, a model of this intersection was created with Synchro 8. The results of this model show that in the AM peak hour, the eastbound movement experiences 7 seconds of delay while the westbound movement experiences 6 seconds of delay. In the PM peak hour, the eastbound movement experiences 4 seconds of delay. In the absence of the new traffic signal, as stated in previous sections, delays at the Fidelia signal will increase due to additional westbound U-turn vehicles. The model also shows that the 95th percentile queue during the AM peak hour for the eastbound through movement is 80 feet with 32 feet for the left-turn and the westbound

through movement is 127 feet with 79 feet for the left-turn. During the PM peak hour, the queue for the eastbound through movement is 170 feet with 25 feet for the left-turn and the westbound through movement is 85 feet with 37 feet for the left-turn. The eastbound and westbound left-turn lanes at the new traffic signal will accommodate the 95th percentile queue lengths and will not impact the operation of the Fidelia signal and the ramp signal. Finally, the new traffic signal will eliminate the non-landscaped raised center median in front of Driveway 2 and Driveway 3. However, the westbound left-turn lane can be modified with the implementation of the new traffic signal to provide a 100-foot lane with a 45-foot transition. The transition will increase the amount of raised center median on Washington Boulevard and will allow the installation of additional groundcover and up to 3 palm trees. Therefore, the new traffic signal is warranted, will not significantly impact the operation of Washington Boulevard, will increase the safety of the left-turn movements across three lanes by providing signal control, and will provide an opportunity to enhance the beautification of this arterial in the City of Commerce.



# Attachment 2

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 1 of 5)

Ma			shir	PM ngton 2/Driv	Blvd. veway	3		Critica	HK _	oach	PH Spee	D	ATE _	10/2	<u>0</u> n	
	Speed limit or critical speed on major street traffic > 40 mph															
WARRANT 1 - Eight Hour Vehicular Volume SATISFIED YES ☑ NO ☐ (Condition A or Condition B or combination of A and B must be satisfied)																
Co	ndition A - Mi	nim	um \	<b>Vehicle</b>	<b>Vol</b> ur	ne			100	)% S	ATIS	SFIED	YES		NO	<b>√</b>
					QUIREN IN BRAC				80	)% S	ATIS	SFIED	YES		NO	7
			U	R	U	R	P	₽.	B.	PM	PM	AM	B. B.	AM		
	APPROACH LANES			1	2 or l		5:00	<del>%</del>	4. 8	23. 80.	- 2 2 2 2 2	/ 3/ /-	9:90	9:90	_/	Hour
	Both Approaches Major Street		500 \$00)	350 (280)	(600 (480)	420 (336)	1768	2072	1983	217	4 179	162	5 156	163	0	
	Highest Approach Minor Street		150 120)	105 (84)	200 (160)	140 (112)	182	149	134	134	4 12	8 117	7 115	10	7	
Co	ondition B - Int		MNIN	IUM RE	ontinu QUIREM IN BRAC	1ENTS						SFIED SFIED	YES		NO NO	_
			U	R	U	R	P	PA.	P	Ā	PM	AM	Δ	AM.		
	APPROACH LANES		_	1	2 or l	More	5:00	08:1	9/	2:30	12:00	/ <u>8/</u> /-	06:9	00:90	_/	Hour
	Both Approaches Major Street		750 500)	525 (420)	(900) (720)	630 (504)	1768	2072	1983	217	4 179	162	5 1564	163	0	
	Highest Approach Minor Street		75 60)	53 (42)	100 (80)	70 (56)	182	149	134	134	1 12	8 117	7 115	107	7	
Combination of Conditions A & B SATISFIED YES □ NO ☑																
	REQUIREMEN	Т			(	CONDIT	ION				<b>✓</b>	FU	LFILLI	ΞD		
	TWO CONDITIO	NC	A.	MINIMU	M VEHI	CULAR	VOLU	IME							1	
			AND, B. INTERRUPTION OF CONTINU				IUOUS TRAFFIC				Yes 🗌 1		<b>10 </b>			
	AND, AN ADEQUATE TRIAL OF OTHER ALTERNATIVES THAT COULD CAUSE LESS DELAY AND INCONVENIENCE TO TRAFFIC HAS FAILED YES NO TO SOLVE THE TRAFFIC PROBLEMS															

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 2 of 5)

WARRANT 2 - Four Hour Vehicular	Volume	SATISFIED*	YES 🗹	NO □
Record hourly vehicular volumes for any for	_ ' ' _ /	, ,		
APPROACH LANES	One More to A	Hour		
Both Approaches - Major Street	1768 2072 1983	2174		
Higher Approach - Minor Street	182 149 134	134		
*All plotted points fall above the applicabl	e curve in Figure 4C-1. (URBAN	AREAS)	Yes 🗸	No 🔲
OR, All plotted points fall above the applic	cable curve in Figure 4C-2. (RUI	RAL AREAS)	Yes 🔲	No 🔲
WARRANT 3 - Peak Hour (Part A or Part B must be satisfied)	:	SATISFIED	YES 🗹	№ □
PART A (All parts 1, 2, and 3 below must be sat one hour, for any four consecutive 15-		SATISFIED	YES 🗆	NO 🗆
The total delay experienced by traffic or controlled by a STOP sign equals or ex approach, or five vehicle-hours for a two	ceeds four vehicle-hours for a or	e direction only) ne-lane	Yes 🗆	No 🗆
The volume on the same minor street a     100 vph for one moving lane of traffic or			Yes 🗆	No 🗖
The total entering volume serviced during for intersections with four or more approaches.			Yes 🗖	No 🗆
PART B	Wd /	SATISFIED	YES 🗹	NO 🗆
APPROACH LANES	One More ic Hour			
Both Approaches - Major Street	1768			
Higher Approach - Minor Street	<b>√</b> 182			
The plotted point falls above the applicable	e curve in Figure 4C-3. (URBAN	N AREAS)	Yes 🗸	No 🔲
OR, The plotted point falls above the appl	icable curve in Figure 4C-4. (RL	JRAL AREAS)	Yes 🔲	No 🔲

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 3 of 5)

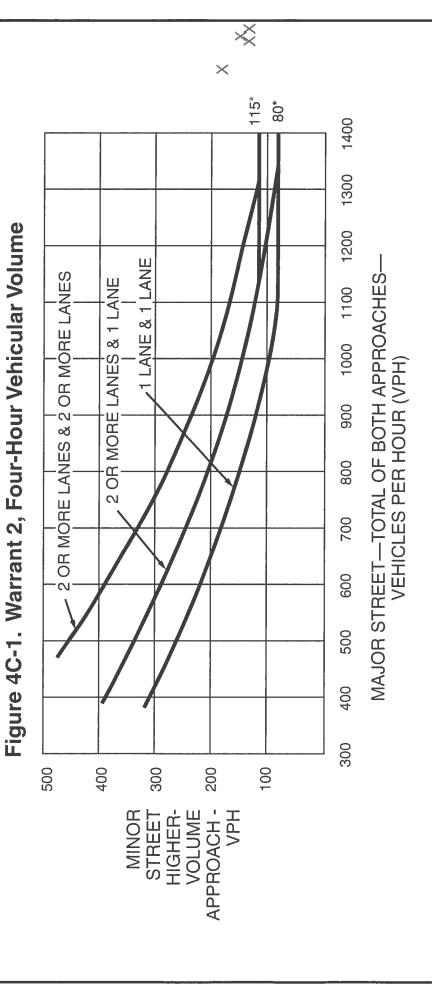
(Parts 1 and 2 Must Be Satisfied)	SATISFIED	YES	NO 🗆
Part 1 (Parts A or B must be satisfied) Hours>			
A. Vehicles per hour for any 4 hours	Figure 4C-5 SATISFIED	_	
Pedestrians per hour for any 4 hours			
Hours>			
B Vehicles per hour for any 1 hour	Figure 4C-7 SATISFIED	_	
Pedestrians per hour for any 1 hour	0		
Part 2	SATISFIED	YES 🗆	по □
AND. The distance to the nearest traffic signal along the major street is githan 300 ft	greater	Yes 🗆	No 🗆
OR, The proposed traffic signal will not restrict progressive traffic flow along	the major street	Yes 🗆	No 🗌
WARRANT 5 - School Crossing (Parts A and B Must Be Satisfied)	SATISFIED	YES 🗆	NO E
(Farts A and D Must be Satisfied)			NO [
Part A Gap/Minutes and # of Children Hour	SATISFIED	YES 🗆	NO 🗆
Part A Gap/Minutes and # of Children  Hour  Gaps Minutes Children Using Crossing	SATISFIED	YES 🗆	
Part A Gap/Minutes and # of Children  Hour  Gaps vs Minutes Children Using Crossing Number of Adequate Gaps  Gaps < M		YES	
Part A Gap/Minutes and # of Children  Hour  Gaps vs Minutes Children Using Crossing Number of Adequate Gaps  Gaps < M			NO 🗆
Part A Gap/Minutes and # of Children  Hour  Gaps vs Minutes Children Using Crossing Number of Adequate Gaps  Gaps < M	linutes dren > 20/hr	YES 🗆	NO
Part A Gap/Minutes and # of Children  Hour  Gaps VS Minutes Children Using Crossing VS Minutes Number of Adequate Gaps School Age Pedestrians Crossing Street / hr  AND Child	linutes dren > 20/hr	YES T	NO
Part A Gap/Minutes and # of Children  Gaps VS Minutes Children Using Crossing VS Minutes Number of Adequate Gaps School Age Pedestrians Crossing Street / hr  AND Child  AND, Consideration has been given to less restrictive remedial measures	linutes dren > 20/hr s. SATISFIED	YES T	NO   NO   NO   NO   NO

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 4 of 5)

WARRANT 6 - Co (All Parts Must Be	ordina Satis	ted Signal System fied)	SA	TISFIED	) Y	ES 🗆	NO 🗹
MINIMUM REQUIRE	MENTS	DISTANCE TO NEAR	EST SIGNAL				
≥ 1000 ft		Nft, Sft, E _	690 ft, W	515 ft		Yes 🗌	No 🗸
traffic control signals vehicular platooning.	are so fa 	et that has traffic predominantly in car apart that they do not provide the	necessary d	egree of 	ent	Yes□	No□
OR, On a two-way st degree of platooning provide a progressive							
WARRANT 7 - Cra (All Parts Must Be	sh Ex Satist	perience Warrant fied)	SA	TISFIED	Y	ES 🗆	NO [
Adequate trial of alter reduce the crash frequency		with satisfactory observance and er	nforcement ha	as failed to	0	Yes 🗌	No
REQUIREMENT	rs	Number of crashes reported within a susceptible to correction by a traffic or damage exceeding the requirement	signal, and in	volving inj		Yes 🗌	No 🗌
5 OR MORE							
REQUIREMENT	S	CONDITIONS	$\checkmark$				
		Warrant 1, Condition A - Minimum Vehicular Volume					
ONE CONDITION SATISFIED 80°		OR, Warrant 1, Condition B - Interruption of Continuous Traffic					No
		OR, Warrant 4, Pedestrian Volume Ped Vol ≥ 152 for any hour OR, Ped Vol ≥ 80 for any 4 hours	e Condition				
WARRANT 8 - Roa (All Parts Must Be			SA	TISFIED	Y	ES 🗆	NO 🗸
MINIMUM VOLUME REQUIREMENTS		ENTERING VOLUMES - ALL API	PROACHES		<b>✓</b>	FULFI	LLED
1000 Veh/Hr	and ha	Typical Weekday Peak Hour is 5-year projected traffic volumes the same of the same o	hat meet one	Veh/Hr or more	<b>/</b>	Yes 7	No
OR During Each of Any 5 Hrs. of a Sat. or Sun Veh/Hr						162 🖸	МОП
CHARACT	ERISTIC	S OF MAJOR ROUTES	MAJOR ROUTE A	MAJOF ROUTE			
	as Princ	sipal Network for Through Traffic	<b>√</b>				
Rural or Suburban Highway O	utside O	f, Entering, or Traversing a City					
Appears as Major Ro	ute on a	n Official Plan	$\checkmark$				
A	ny Maio	Route Characteristics Met. Both Si	treets			Yes 🗆	No 🗸

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 5 of 5)

WARRANT 9 - Intersection Near a Grade Crossing (Both Parts A and B Must Be Satisfied)	SATISFIED	YES NO
PART A		
A grade crossing exists on an approach controlled by a STOP or YIELD center of the track nearest to the intersection is within 140 feet of the st line on the approach. Track Center Line to Limit Line ft	Yes No	
PART B		
There is one minor street approach lane at the track crossing - Du traffic volume hour during which rail traffic uses the crossing, the plotted the applicable curve in Figure 4C-9.		
Major Street - Total of both approaches: VPH Minor Street - Crosses the track (one direction only, approaching the in VPH X AF (Use Tables 4C-2, 3, & 4 below to calculate AF) =	. — Yes □ No □	
OR, There are two or more minor street approach lanes at the trace During the highest traffic volume hour during which rail traffic uses the other plotted point falls above the applicable curve in Figure 4C-10.		
Major Street - Total of both approaches : VPH Minor Street - Crosses the track (one direction only, approaching the in VPH X AF (Use Tables 4C-2, 3, & 4 below to calcualte AF) = _		
The minor street approach volume may be multiplied by up to three follows as described in Section 4C.10.	ng adjustment facto	ors (AF)
1- Number of Rail Traffic per Day	Adjustment factor i	from table 4C-2
2- Percentage of High-Occupancy Buses on Minor Street Approach	from table 4C-3	
3- Percentage of Tractor-Trailer Trucks on Minor Street Approach	from table 4C-4	
NOTE: If no data is availale or known, then use AF = 1 (no adjustment)		



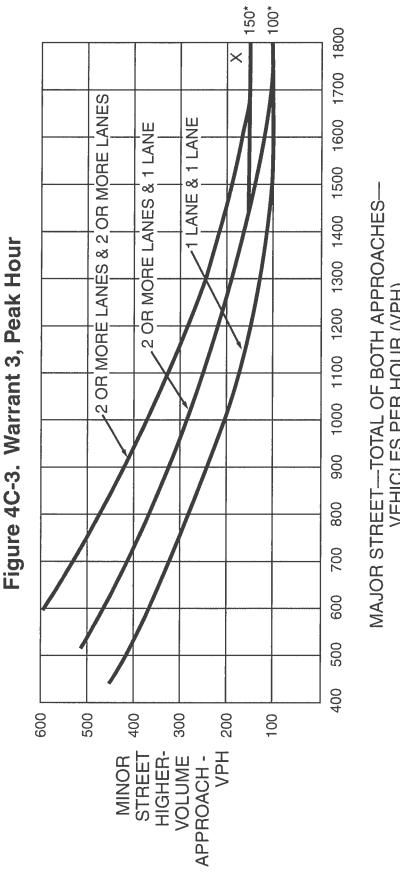
\*Note: 115 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 80 vph applies as the lower threshold volume for a minor-street approach with one lane.

0/26/16

Warrant 2

Existing Volumes with Street Improvements
Washington Blvd. and Driveway 2/Arco Driveway





# VEHICLES PER HOUR (VPH)

\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

10/26/16

Washington Blvd. and Driveway 2/Arco Driveway Existing Volumes with Street Improvements Warrant 3



